

The evolution of slope failures: mechanisms of rupture propagation

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Abstract. Forecasting the occurrence of large, catastrophic slope failures remains very problematic. It is clear that in order to advance this field a greater understanding is needed of the processes through which failure occurs. In particular, there is a need to comprehend the processes through which a rupture develops and propagates through the slope, and the nature of the inter-relationship between the stress and strain states of the landslide mass. To this end, a detailed analysis has been undertaken of the movement records for the Selborne Cutting slope failure, in which failure was deliberately triggered through pore pressure elevation. The data demonstrate that it is possible to determine the processes occurring in the basal region of the landslide, and thus controlling the movement of the mass, from the surface movement patterns. In particular, it is clear that the process of rupture development and propagation has a unique signature, allowing the development of the rupture to be traced from detailed surface monitoring. For landslides undergoing first time failure through rupture propagation, this allows the prediction of the time of failure as per the “Saito” approach. It is shown that for such predictions to be reliable, data from a number of points across the landslide mass are needed. Interestingly, due to the complex stress regime in that region, data from the crown may not be appropriate for failure prediction.

Based upon these results, the application of new techniques for the detailed assessment of spatial patterns of the development of strain may potentially allow a new insight into the development of rupture surfaces and may ultimately permit forecasting of the temporal occurrence of failure.

1 Introduction

At present our understanding of the processes occurring within the basal region of a landslide is relatively poor on both the macro- and micro-scale. These processes are impor-

tant as in most landslides occurring in intact materials (i.e. in the form of translation or rotational slides), it is the basal processes that control the surface movement patterns. Thus, the development of an understanding of the processes occurring within the basal zone might pave the way to models that allow prediction of the future patterns of movement of a landslide.

This paper examines the development of one particular failure, based upon a detailed data set collected as part of the Selborne Cutting Slope experiment in 1989. Analysis has been conducted on the movement record as recorded by inclinometers located within the landslide mass. The manner in which the failure developed and propagated through the landslide mass is described using the velocity – time method of Petley et al. (2002). From this analysis, some general conclusions may be drawn about the development of failure in landslide systems, and insight has been obtained into the optimal monitoring approaches for the prediction of the time of failure of landslides.

2 Accelerating trends in landslides

In recent years there has been renewed interest in the understanding of basal deformation processes. This has been spurred by a reassessment of the observations made initially by Saito (1988) and developed in more detail by Voight (1988, 1989) and Fukuzono (1990). Here, it was observed that failures in many landslides are preceded by an accelerating trend that, when plotted in $\Lambda-t$ space, where $\Lambda=1/v$ and v = velocity, and t is time, yield a straight line. Although this observation has been used by various authors (Hutchinson, 2001 for example), further development of the theory did not occur until Kilburn and Petley (2003) derived a mathematical expression to characterise accelerating behaviour. They noted that linearity is probably associated with crack growth, suggesting that linearity should be noted in materials in which brittle failure was the dominant mechanism. Where the process occurring at depth is ductile, or indeed consists

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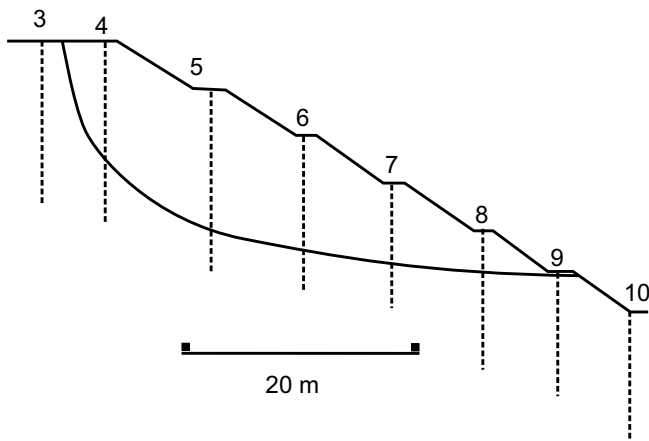


Fig. 1. Cross section through the Selborne landslide, indicating the location of the inclinometers discussed in this paper (after Cooper et al., 1998). No vertical exaggeration.

of sliding on existing surfaces, they suggested that an asymptotic trend might be expected to be observed in $\Lambda-t$ space. This is borne out by the work of Petley et al. (2002), who demonstrated that an asymptotic trend is indeed displayed in non-brittle landslide systems. Further, Petley and Petley (2004) demonstrated that for the massive, catastrophic Vajont landslide in 1963 two periods of acceleration prior to the final failure were not characterised by linearity, whilst the final failure did show a clear linear trend. This was taken to imply that the basal mechanisms evolved through time, changing from an essentially ductile process during the creeping phase to a brittle process at final failure. This concurs with the suggestions made on the basis of the deformation mechanics by Petley (1996) and Petley and Allison (1997).

However, there is much to learn about the evolution of failure in landslide systems and in particular about the ways in which fracture propagation through the basal zone of the landslide allows the development of the final failure. This is a key aspect in the understanding of the development of landslides, and in particular in the prediction of the time of final failure and the evolution of the basal region of a landslide system.

3 Selborne Cutting Slope experiment

The remarkable Selborne Cutting Slope experiment of 1989, described in detail in Cooper et al. (1998), undoubtedly still provides the best available data set regarding the development of a first time failure. This landslide was intentionally triggered in a large slope cut into the side of an old brickpit in Hampshire, England. The failure was triggered in a 1:2 gradient benched slope in Gault Clay through careful and controlled increases in pore pressure generated through artificial groundwater recharge using a network of specially-constructed wells and a constant head water tank system. The slope was instrumented with a comprehensive network

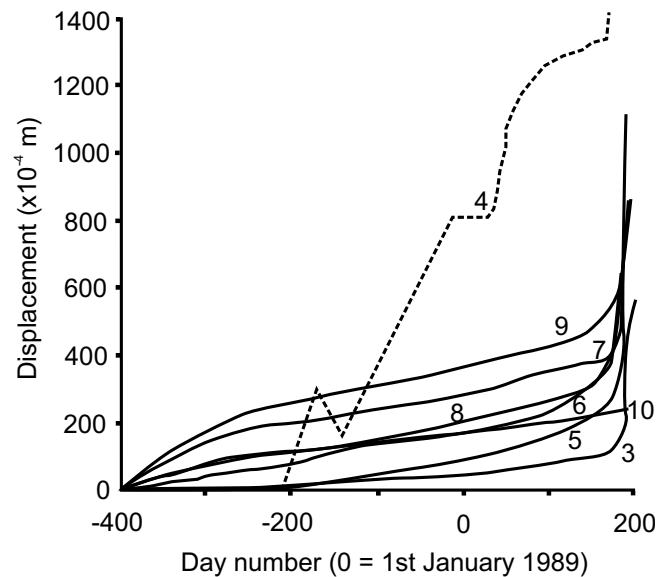


Fig. 2. Displacements of the landslide surface plotted against time, as determined from the inclinometer readings. Day 0 is the time at which pore pressure recharge was initiated.

of piezometers, inclinometers and extensometers, providing a full data set of the slope processes prior to and during the failure event.

The failure was triggered in a highly controlled manner. After the initial slope cutting the system was given a long period to equilibrate prior to groundwater recharge. Groundwater levels were elevated over a period of some 180 days until failure was triggered. The failure occurred in the form of a slump with a curved back scar. Movement developed in a sedate manner, with maximum recorded displacement rates of 58 mm/h (Cooper et al., 1998).

4 Displacement records

The full instrument data set from the Selborne experiment are publicly available in electronic form from the project team. In this study, detailed analysis has been undertaken of the surface displacements of the Selborne slope as determined from the inclinometer records. Twelve inclinometers were emplaced within the slope, of which six were situated within the landslide (Fig. 1). This study examines the records of these eight, plus one located immediately above the crown of the landslide, and one just below the toe (Fig. 1). An analysis of the movement pattern of the landslide as derived from the 24-h inclinometer data in the vicinity of the shear surface was presented by Cooper et al. (1998), who noted that:

- Initial displacements were noted in the vicinity of the eventual shear surface from the start of the experiment, prior to pore pressure recharge being initiated. This was especially the case in the vicinity of the toe of the landslide.

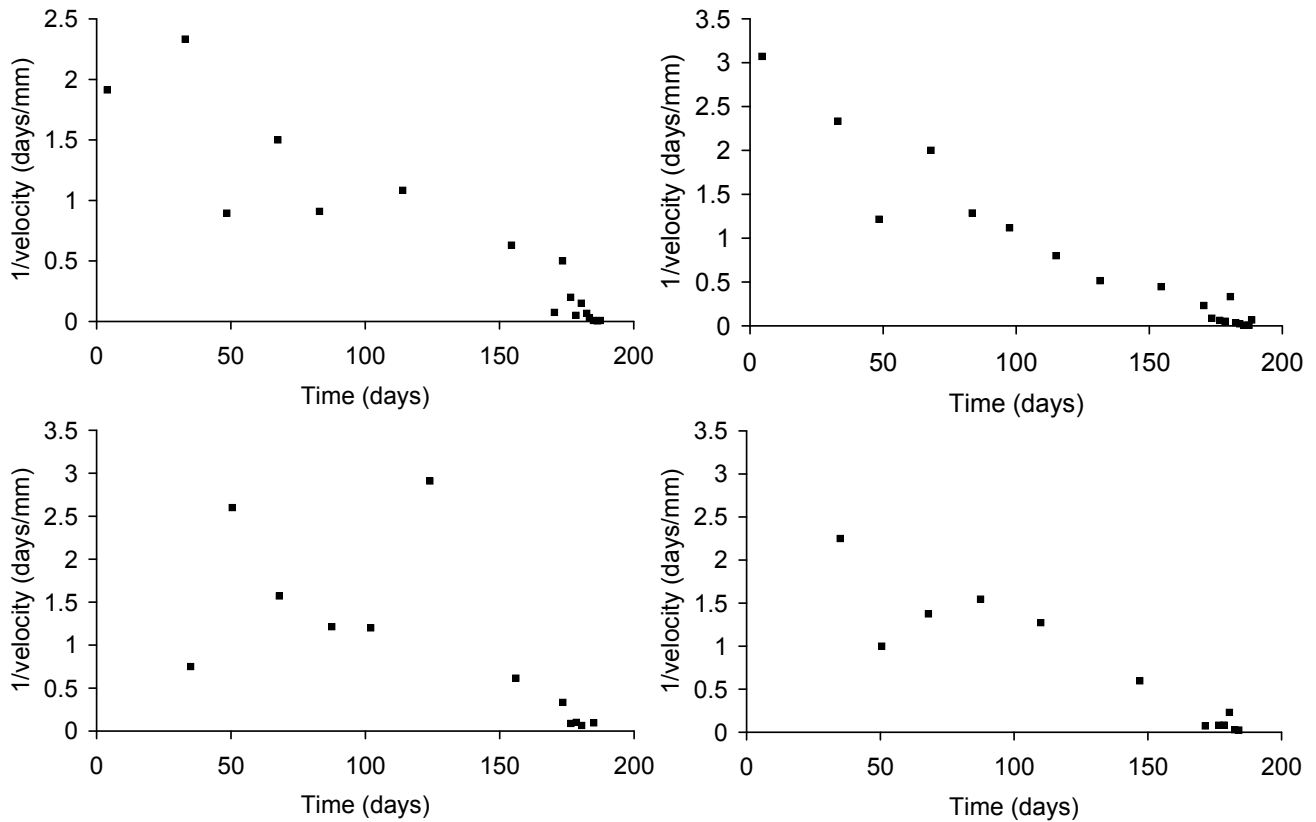


Fig. 3. Displacements of the landslide surface plotted in $\Lambda-t$ space: (a) Inclinometer 5; (b) Inclinometer 6; (c) Inclinometer 7; (d) Inclinometer 8.

- For the failure event, the inclinometers in the vicinity of the centre of the slope (inclinometers 5 and 6) showed very little movement prior to about day 160, but then showed a rapid and dramatic acceleration to final failure.
- Inclinometer 4, which was situated close to the rear scarp, displayed a behaviour that is in considerable variance to all of the other instruments. The causes of this were not discussed.

The nature of the accelerating trends for these inclinometers in the vicinity of the shear surface was analysed by Petley et al. (2002), who noted that when plotted in $\Lambda-t$ space, inclinometer 8 showed a clear linear trend in the 40 days prior to final failure. Whilst they acknowledged that there is the possibility of some late-stage steepening of the line, analysis of the data using the techniques of Fukuzono (1990) allows forecasting of the time of final failure to within ± 4 days. Similar linearity was also been noted for the inclinometers located upslope, although the early movements were slower and there is more scatter in the data. This was taken to imply that the early phases of movement were dominated by internal deformations, whereas the late-stage rapid acceleration suggests the propagation of the fracture upslope.

Thus, Petley et al. (2002) provisionally concluded that the linear trend appears to represent movement in different

parts of the landslide associated with fracture propagation, which is in agreement with the post-failure analyses of the site and geotechnical investigations by Cooper et al. (1998), who clearly demonstrated that the final failure occurred as a result of the development of a discrete shear surface.

Petley et al. (2002) proposed that the $\Lambda-t$ approach offers the potential for determining the type of deformation that is occurring in the basal region of a landslide and, where that deformation is occurring in the brittle manner, and thus the $\Lambda-t$ plot is linear, to predict the likely time of catastrophic failure. To maximise its usefulness in the actual prediction of the time of failure the technique must be applicable to surface instrument readings. This is because these readings are the most easily obtained with a high temporal frequency, and are the least prone to equipment failure. Figure 3 presents the $\Lambda-t$ plots for the surface deformations greater than 0.1 mm/day for inclinometers 5, 6, 7 and 8, which were the four instruments located within the main body of the landslide. In three of the four cases (inclinometers 5, 6 and 8) the plots display a clearly linear trend based on 24-h data, although in all cases the scatter in the data is substantial for the first 70 days. This scatter is inevitable for a Selborne type landslide in which movement rates are very slow – during the first 70 days movement occurred at a rate in the order of 0.3–1.0 mm/day. These movement rates are close to the resolution limits of the instruments. As the

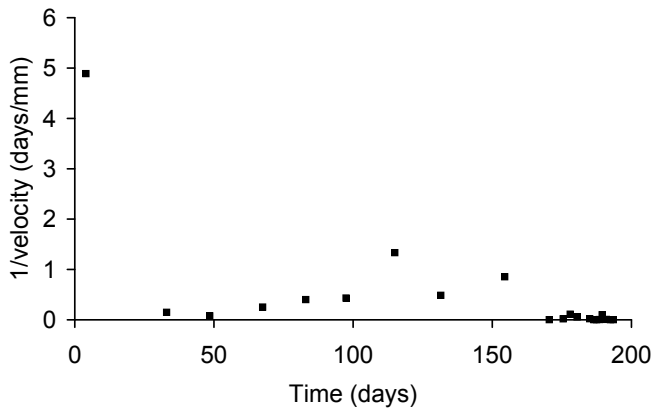


Fig. 4. Displacements of the landslide surface for Inclinometer 4, plotted in $\Lambda-t$ space.

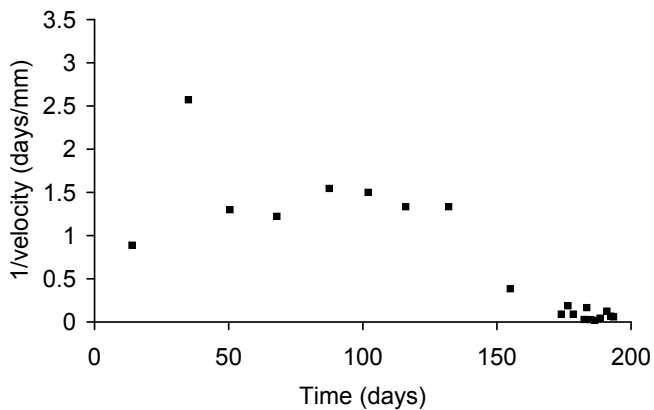


Fig. 5. Displacements of the landslide surface for Inclinometer 9, plotted in $\Lambda-t$ space.

velocity increases so the data quality improves. More than a decade after the experiment it is probably now possible to install instruments with a much higher resolution, avoiding some of these issues.

The instrument that shows linearity from the very start of the experiment is Inclinometer 6 (Fig. 3b), located in the very middle of the landslide. Petley et al. (2002) and Petley and Petley (2004) demonstrated that the onset of linearity for any part of the landslide mass is associated with the effects of crack propagation affecting that zone. It is thus logical to suggest that crack initiation started in the vicinity of Inclinometer 6 and then propagated outwards. It is difficult to determine when linearity occurs for the other instruments, but the greatest scatter occurs for inclinometer 7, perhaps suggesting that propagation was initially dominantly upslope (i.e. towards inclinometer 5), and then downslope (towards inclinometer 7 and 8) from about day 80. Note that high levels of scatter continue to occur in instrument 7 until about day 150.

The deformation data of inclinometer 4 are more difficult to interpret (Fig. 4). The displacement – time plot shows that

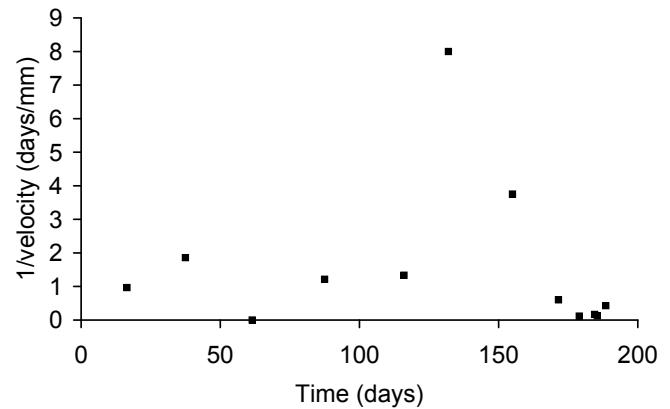


Fig. 6. Displacements of the landslide surface for Inclinometer 10, plotted in $\Lambda-t$ space.

this inclinometer indicates the most rapid surface movements of any of the instruments, and that these movements were initiated very early in the experiment and then accelerated in an unpredictable manner (Fig. 2). The final rapid movements do coincide with the rapid movements of the other instruments.

When pore pressure recharge was initiated this area underwent an abrupt acceleration, followed by a period of gradual deceleration. Very late on this area accelerated abruptly once again. Unsurprisingly this yields essentially uninterpretable data in $\Lambda-t$ space (Fig. 4). Some semblance of linearity is possibly indicated late on, but this may just be an artefact. The reasons why this point should show such different behaviour are unclear at this point. Although Cooper et al. (1998) do not suggest this, it is possible that this section was behaving initially as a separate system. Alternatively, perhaps the proximity of the steep back-scarp, and the resultant tensile stresses that occur in this region, mean that the deformation pattern is quite different. Either way, the movement data for this point suggest that monitoring in the vicinity of the back scarp may not allow an analysis of the behaviour of the basal materials or the prediction of the time of final failure where appropriate.

Inclinometer 9 was located in the vicinity of the toe of the landslide, but just within the mobile mass. Interestingly, in the early phases of the experiment, soon after cutting, quite substantial movements were noted. It is notable however that the accelerating phase occurred later than for the parts of the landslide situated upslope (Fig. 2). This is reflected in the plot in space (Fig. 5), with an initial high level of scatter, settling down to an approximately constant value of Λ , before a late stage acceleration at about day 135. Thus, although some early deformation occurred in this area, propagation of the fracture to this point appears to have occurred late in the experiment.

Inclinometer 10 was situated below the toe of the main landslide. During the experiment it showed some creep (Fig. 2), with a slight acceleration during the final failure of the main landslide mass. Plotted in $\Lambda-t$ space (Fig. 6), the data indicates no accelerating trend that can be relied upon,

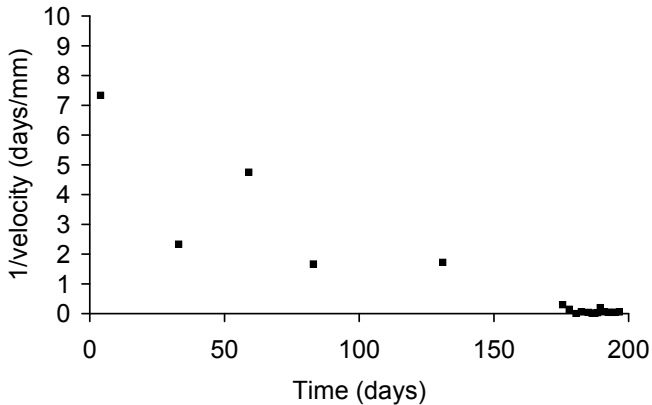


Fig. 7. Displacements of the landslide surface for Inclinometer 3, plotted in $\Lambda-t$ space.

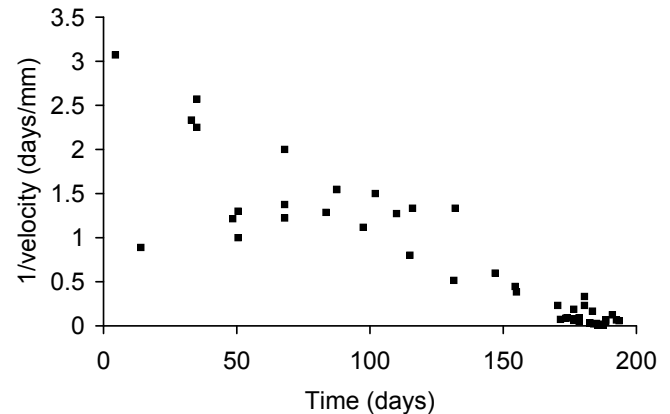


Fig. 8. Displacements of the landslide surface for Inclinometers 6, 8 and 9 combined, plotted in $\Lambda-t$ space.

showing that off the landslide very different effects are seen. Similar results are seen for Inclinometer 3 (Fig. 7), located above the back scarp of the failure, which although displaying some sort of accelerating trend, does not produce a statistically valid result.

Thus, in general the inclinometers in the main body of the landslide indicate that linearity can be determined in this area. However, there is considerable scatter in the data. Substantial improvements in this are noted by combining the data from a number of instruments in this area (Fig. 8). This combined data set clearly shows the linear trend, with relatively low levels of data scatter in the final 70 days or so. The time of the final failure could have been predicted with a reasonable level of confidence (i.e. ± 5 days) approximately 50 days in advance of its actual occurrence.

Thus, it appears that a monitoring programme for the interpretation of landslide movement mechanisms and the prediction of the time of final failure should attempt to examine the movement patterns of a range of points within the potential landslide mass, rather than just one. Thus, combined readings from a number of extensometers or inclinometers would appear to be ideal. Alternatively, the application of interferometric techniques, which measure the deformation of many points on the ground surface (Bulmer et al., 2002; Tarchi et al., 2003) or total station techniques (Mantovani et al., 2000) would appear to be ideal, although in the latter case the technology has yet to mature fully.

5 Discussion

The excellent Selborne data set collected by Cooper et al. (1998) provides important insights into the process by which first time failures develop. The data show that the development of failure is initiated in the midslope area and then propagates both up and downslope to allow final failure to occur. However, the Selborne experiment should be considered to be an artificial landslide that was relatively small

in extent. Therefore, a critical question is to what degree the processes observed in this slope can be extrapolated to large, natural systems? In fact there is quite good evidence that these processes do indeed occur in natural first time failures. There are several pieces of evidence to support this. First, Petley and Petley (2004) have demonstrated that the extremely large Vajont landslide displayed linearity prior to the catastrophic failure. Second, many other natural landslides over a wide range of scales have also shown the linear trend (see for example Petley et al., 2002). Thirdly, Kilburn and Petley (2003) showed using first principles that linearity is a fundamental process associated with crack growth in natural systems. Finally, it should be noted that whilst the Selborne failure was an artificially induced landslide, the project team tried to replicate natural processes as much as possible by, for example, inducing failure through controlled pore pressure increases and by ensuring that the slope was formed from undisturbed materials.

6 Conclusions

This study has demonstrated that the $\Lambda-t$ approach can yield substantial amounts of information about the mechanisms and future behaviour of a landslide. For the Selborne landslide, for which the movement data are very well-constrained, it is clear that the rupture was initiated in the midst of the landslide and then propagated outwards. Linearity is clearly noted for the surface displacement records for points located on the main landslide mass, although the displacements of a point located in the vicinity of the back scarp are difficult to interpret. The best interpretations of the likely future behaviour of the landslide can be made through the combined analysis of a number of points located in the main body of the landslide. This offers real hope for the development of systems that permit the prediction of the future behaviour of unstable slopes.

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References

- Bulmer, M. H., Mantovani, F., Murphy, W., and Petley, D. N.: SAR, Interferometry in landslide monitoring; experiences from Black Ven Landslide, Dorset 2001, in: *Geomorphology: From Expert Opinion to Modelling*, European Centre on Geomorphological Hazards, edited by Delahaye, D., Levoy, F., and Maquaire, O., Strasbourg, France, 21–27, 2002.
- Cooper, M. R., Bromhead, E. N., Petley, D. J., and Grant, D. I.: The Selborne cutting stability experiment, *Geotechniq.*, 48, 83–101, 1998.
- Fukuzono, T.: Recent studies on time prediction of slope failure, *Landslide News*, 4, 9–12, 1990.
- Hutchinson, J.: Landslide risk – to know, to foresee, to prevent, *Geologia Tecnica and Ambientale*, 9, 3–24, 2001.
- Kilburn, C. R. J. and Petley, D. N.: Forecasting giant, catastrophic slope collapse: Lessons from Vajont, northern Italy, *Geomorpho.*, 54, 49–62, 2003.
- Mantovani, F., Pasuto, A., Silvano, S., and Zannoni, A.: Collecting data to define future hazard scenarios of the Tessina landslide, *ITC Journal*, 1, 33–40, 2000.
- Petley, D. N.: The mechanics and landforms of deep-seated landslides, in: *Brookes, S. and Anderson, M. G., Advances in hillslope processes*, John Wiley & Sons, UK, 823–835, 1996.
- Petley, D. N. and Allison, R. J.: The mechanics of deep-seated landslides, *Earth Surf.*, 22, 747–758, 1997.
- Petley, D. N., Bulmer, M. H. K., and Murphy, W.: Patterns of movement in rotational and translational landslides, *Geology*, 30, 719–722, 2002.
- Petley, D. N. and Petley, D. J.: On the initiation of large rock-slides: perspectives from a new analysis of the Vaiont movement record, in: *Large Rock Slope Failures*, edited by Paper Evans, S., Balkema, Rotterdam (NATO Science Series), accepted, 2004.
- Saito, M.: Semi-logarithmic representation for forecasting slope failure, *Proceedings of the International Symposium on Landslides*, 1, 321–324, 1980.
- Tarchi, D., Casagli, N., Fanti, R., Leva, D. D., Luzi, G., Pasuto, A., Pieraccini, M., and Silvano, S.: Landslide monitoring by using ground-based SAR interferometry: an example of application to the Tessina landslide in Italy, *Eng. Geol.*, 68, 15–30, 2003.
- Voight, B.: A relation to describe rate-dependent material failure, *Science*, 243, 200–203, 1988.
- Voight, B.: A method for prediction of volcanic eruptions, *Nature*, 332, 125–130, 1989.